

For a given rectangular concrete beam, according to Eurocode 1992-1-1:

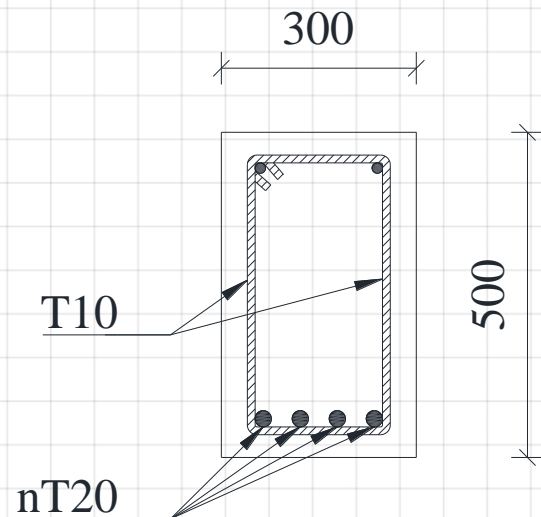
- a) Derive the procedure of determining the required reinforcement.

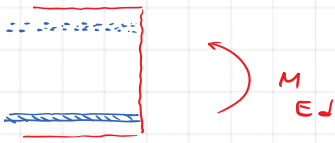
A concrete rectangular cross-section,  $300 \times 500$ , is made of C30/37 and reinforced by B500 steel. Assume the stirrup is T10 and the required nominal cover is 35mm:

- b) If the available reinforcement is T20, how many reinforcements are required to withstand  $250 \text{ kN.m}$  bending moment?  
c) Determine the stress in reinforcements based on your design.

PS. Neglect the effect of compressive steel.

Dimensions are in mm. The cross-section is under a positive bending moment. The sketch is schematic and does not necessarily mean that 4T20 is sufficient.





$$M_{Rd} = \mu \cdot f_{cd} \cdot b \cdot d^2$$

$$\mu = \beta \left(1 - \frac{\beta}{2}\right)$$

$$\omega = \frac{A_s}{b \cdot d} \cdot \frac{f_{td}}{f_{cd}}, \quad \beta_{bd} = 0.493 \quad (f_{yk} = 500 \text{ MPa})$$

$$M_{Rd,c} = \mu_{bd} \cdot f_{cd} \cdot b \cdot d^2$$

$$\mu_{bd} = \beta_{bd} \left(1 - \frac{\beta_{bd}}{2}\right) = 0.493 \left(1 - \frac{0.493}{2}\right) = 0.372$$

$$M_{Ed} = 250 \text{ kNm} < M_{Rd,c} = 300 \text{ kNm} \rightarrow \text{Tensile reinforcement would be enough.}$$

$$M_{Ed} = 400 \text{ kNm} > M_{Rd,c} = 300 \text{ kNm} \rightarrow \begin{cases} \text{using better concrete class} \\ \text{increasing beam dimensions} \rightarrow \begin{cases} b \\ h \end{cases} \\ \text{we can check if by} \\ \text{reasonable compressive reinforcement} \\ \text{the beam can take } M_{Ed} \end{cases}$$

① Determine the  $M_{Rd,c}$  (Maximum bending moment capacity of concrete beam without compressive reinforcement)

② If  $M_{Ed} < M_{Rd,c} \rightarrow$  Compressive reinforcement is not needed.

③  $M_{Ed} = \mu \cdot f_{cd} \cdot b \cdot d^2 \rightarrow \mu = \frac{M_{Ed}}{f_{cd} \cdot b \cdot d^2}, \quad (\mu < \mu_{bd})$

$$\begin{aligned} \mu &= \beta \left(1 - \frac{\beta}{2}\right) \Rightarrow \beta^2 - 2\beta + 2\mu = 0 \\ \mu &= \beta - \frac{\beta^2}{2} \\ 2\mu &= 2\beta - \beta^2 \end{aligned}$$

$$\beta = \frac{2 \pm \sqrt{4 - 8\mu}}{2} = \frac{2 \pm 2\sqrt{1 - 2\mu}}{2} = 1 \pm \sqrt{1 - 2\mu} \Rightarrow$$

④  $\beta = 1 - \sqrt{1 - 2\mu}$

$$\beta = \frac{A_s}{b \cdot d} \cdot \frac{f_{td}}{f_{cd}} = \frac{\lambda}{d \cdot \frac{\lambda x}{\lambda x}} = \frac{\lambda x}{d} \quad c_c = f_{cd} \cdot b \cdot \lambda x \rightarrow f_{cd} \cdot b = \frac{c_c}{\lambda x}$$

$$\textcircled{5} \quad \beta = \frac{A_s}{b \cdot d} \cdot \frac{f_{td}}{f_{cd}} \Rightarrow A_s = \dots \rightarrow A_{s, \text{required}}$$

$$A_{s, \text{min}} = \max \left\{ 0.26 \frac{f_{ctm}}{f_{yd}}, 0.0013 \right\} \cdot b \cdot d$$

Strength classes for concrete														Analytical relation / Explanation	
$f_{ck}$ (MPa)	12	16	20	25	30	35	40	45	50	55	60	70	80	90	
$f_{ck, \text{cube}}$ (MPa)	15	20	25	30	37	45	50	55	60	67	75	85	95	105	
$f_{ctm}$ (MPa)	20	24	28	33	38	43	48	53	58	63	68	78	88	98	$f_{ctm} = f_{ck} + 8$ (MPa)
$f_{ctm}$ (MPa)	1.6	1.9	2.2	2.6	2.9	3.2	3.5	3.8	4.1	4.2	4.4	4.6	4.8	5.0	$f_{ctm} = 0.30 \cdot f_{ck}^{(2/3)} \leq C50/60$ $f_{ctm} = 2.12 \cdot \ln(1 + (f_{ctm}/10)) > C50/60$

$$A_s = \max \{ A_{s, \text{required}}, A_{s, \text{min}} \}$$

$$\phi_s = ? , n = \dots$$

Width of the beam	$b := 300 \text{ mm}$
Height of the beam	$h := 500 \text{ mm}$
Nominal cover	$c_{nom} := 35 \text{ mm}$
Reinforcement Diameter	$\phi_s := 20 \text{ mm}$
Stirrup Diameter	$\phi_{sw} := 10 \text{ mm}$
Design Bending Moment	$M_{Ed} := 250 \text{ kN} \cdot \text{m}$
Concrete Compressive Strength	$f_{ck} := 30 \text{ MPa}$

$$A_{s, \text{reqd}} = 1520 \text{ mm}^2$$

$$A_{s, \text{provided}} = 5T20 = \frac{5 \times \pi \times (20 \text{ mm})^2}{4}$$

$$A_{s, \text{pr}} = 1571 \text{ mm}^2$$

$$f_{sd} = \frac{1520 \text{ mm}^2}{1571 \text{ mm}^2} \cdot f_{td} = 421 \text{ MPa}$$

$\uparrow$  435 MPa

Status<sub>1</sub> = "Compressive Reinforcement is not Required"

$$A_s = 1520 \text{ mm}^2$$

Required Reinforcement (Number)

$$n_{s, \text{reqd}} = 4.84$$

$$n := 5$$