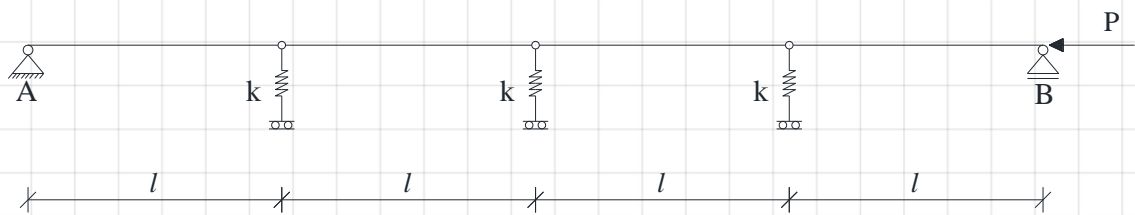
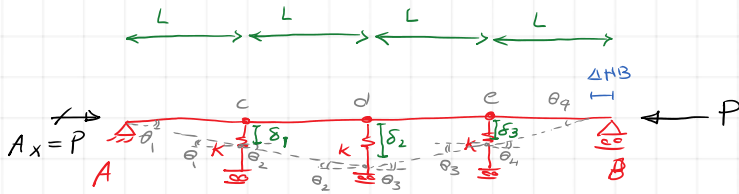


Four rigid bars form a compressive element under applied compressive load at point B. It is assumed that the bars are hinged and connected at their intersection. At one end, the element is supported by a hinge, and at the other end, point B, the element is supported by a roller. The system is assumed to be a 2D structure. At the intersections, there are three transitional springs with a spring coefficient of  $k$ . Each rigid bar is the length of  $l$ .

- Determine the total potential energy of the system.
- Determine the buckling loads.
- Determine the buckling shape modes.
- If the undrained shear strength of soil is  $10kPa$ , determine the buckling load numerically.
- For three Circular Hollow Sections (CHS<sup>1</sup>), determine the Euler buckling load and check which buckling mode would occur first.
- Model the system in FEM software and compare your results.



<sup>1</sup> CHS88,9/6,3, CHS139,7/6,3, and CHS168,3/10



$$\Delta HB = L - L \cos \theta_1 + L - L \cos \theta_2 + L - L \cos \theta_3 + L - L \cos \theta_4$$

$$\Delta HB = L [1 - \cos \theta_1 + 1 - \cos \theta_2 + 1 - \cos \theta_3 + 1 - \cos \theta_4]$$

$$\Delta HB = L \left[ \frac{\theta_1^2}{2} + \frac{\theta_2^2}{2} + \frac{\theta_3^2}{2} + \frac{\theta_4^2}{2} \right]$$

$$\Delta HB = \frac{L}{2} \left[ \left( \frac{\delta_1}{L} \right)^2 + \left( \frac{\delta_2 - \delta_1}{L} \right)^2 + \left( \frac{\delta_2 - \delta_3}{L} \right)^2 + \left( \frac{\delta_3}{L} \right)^2 \right]$$

$$\Delta HB = \frac{L}{2L} \left[ \delta_1^2 + \delta_2^2 + \delta_3^2 - 2\delta_1\delta_2 + \delta_2^2 + \delta_3^2 - 2\delta_2\delta_3 + \delta_3^2 \right]$$

$$\Delta HB = \frac{1}{L} \left[ \delta_1^2 + \delta_2^2 + \delta_3^2 - \delta_1\delta_2 - \delta_2\delta_3 \right]$$

$$V = -P \cdot \Delta HB = -\frac{P}{L} \left[ \delta_1^2 + \delta_2^2 + \delta_3^2 - \delta_1\delta_2 - \delta_2\delta_3 \right]$$

$$W = \sum \frac{1}{2} k \cdot x^2 = \frac{1}{2} \left[ k\delta_1^2 + k\delta_2^2 + k\delta_3^2 \right] = \frac{k}{2} \left[ \delta_1^2 + \delta_2^2 + \delta_3^2 \right]$$

$$\Pi(\delta_1, \delta_2, \delta_3) = W + V = \frac{k}{2} \left[ \delta_1^2 + \delta_2^2 + \delta_3^2 \right] - \frac{P}{L} \left[ \delta_1^2 + \delta_2^2 + \delta_3^2 - \delta_1\delta_2 - \delta_2\delta_3 \right]$$

$$\frac{\partial \Pi}{\partial \delta_1} = 0, \quad \frac{\partial \Pi}{\partial \delta_2} = 0, \quad \frac{\partial \Pi}{\partial \delta_3} = 0$$

$$\frac{\partial \Pi}{\partial \delta_1} = \frac{k}{2} [2\delta_1] - \frac{P}{L} [2\delta_1 - \delta_2] = 0 \Rightarrow \left\{ \begin{array}{l} (k - \frac{2P}{L})\delta_1 + \frac{P}{L}\delta_2 + 0\delta_3 = 0 \\ \frac{P}{L}\delta_1 + (k - \frac{2P}{L})\delta_2 + \frac{P}{L}\delta_3 = 0 \\ 0\delta_1 + \frac{P}{L}\delta_2 + (k - \frac{2P}{L})\delta_3 = 0 \end{array} \right.$$

$$\frac{\partial \Pi}{\partial \delta_2} = \frac{k}{2} [2\delta_2] - \frac{P}{L} [2\delta_2 - \delta_1 - \delta_3] = 0$$

$$\frac{\partial \Pi}{\partial \delta_3} = \frac{k}{2} [2\delta_3] - \frac{P}{L} [2\delta_3 - \delta_2] = 0$$

$$\begin{cases} (k - \frac{2P}{L})\delta_1 + \frac{P}{L}\delta_2 + 0\delta_3 = 0 \\ \frac{P}{L}\delta_1 + (k - \frac{2P}{L})\delta_2 + \frac{P}{L}\delta_3 = 0 \\ 0\delta_1 + \frac{P}{L}\delta_2 + (k - \frac{2P}{L})\delta_3 = 0 \end{cases}$$

$$\underbrace{\begin{bmatrix} k - \frac{2P}{L} & \frac{P}{L} & 0 \\ \frac{P}{L} & k - \frac{2P}{L} & \frac{P}{L} \\ 0 & \frac{P}{L} & k - \frac{2P}{L} \end{bmatrix}}_A \begin{bmatrix} \delta_1 \\ \delta_2 \\ \delta_3 \end{bmatrix} = \begin{bmatrix} 0 \\ 0 \\ 0 \end{bmatrix}$$

$$\det(A) = 0 \Rightarrow$$

$$\det(A(k, p, l)) \xrightarrow[\text{expand}]{\text{simplify}} -\frac{4 \cdot p^3}{l^3} + \frac{10 \cdot k \cdot p^2}{l^2} - \frac{6 \cdot k^2 \cdot p}{l} + k^3 = 0$$

$$-4 \left(\frac{P}{kL}\right)^3 + 10 \left(\frac{P}{kL}\right)^2 - 6 \left(\frac{P}{kL}\right) + 1 = 0 \quad \frac{P}{kL} = x$$

$$-4x^3 + 10x^2 - 6x + 1 = 0 \Rightarrow x =$$

$$-4x^3 + 10x^2 - 6x + 1 = 0 \xrightarrow[\text{float}]{\text{solve, } x} \begin{bmatrix} 0.5 \\ 1.7071067811865475244 \\ 0.2928932188134524756 \end{bmatrix}$$

$$\left(\frac{P}{kL}\right)_1 = 0.2929 \Rightarrow P_{cr1} = 0.2929 kL$$

$$P_{cr2} = 0.5 kL$$

$$P_{cr3} = 1.7071 kL$$

Mathcad  $\Rightarrow$

$$\det(A(k, p, l)) = 0 \xrightarrow[\text{assume, } k > 0, l > 0]{\text{solve, } p, \text{ substitute, } (k^2 \cdot l^2)^{0.5} = k \cdot l} \begin{bmatrix} 0.5 \cdot k \cdot l \\ 1.7071067811865475244 \cdot k \cdot l \\ 0.2928932188134524756 \cdot k \cdot l \end{bmatrix}$$

$$P_{cr1} = 0.2929 KL,$$

$$\begin{bmatrix} k - \frac{2P}{L} & \frac{P}{L} & 0 \\ \frac{P}{L} & k - \frac{2P}{L} & \frac{P}{L} \\ 0 & \frac{P}{L} & k - \frac{2P}{L} \end{bmatrix} \begin{bmatrix} \delta_1 \\ \delta_2 \\ \delta_3 \end{bmatrix} = \mathbf{0} \Rightarrow \begin{bmatrix} k - 2 \times 0.2929k & 0.2929k & 0 \\ 0.2929k & 0.4142k & 0.2929k \\ 0 & 0.2929k & 0.4142k \end{bmatrix} \begin{bmatrix} \delta_1 \\ \delta_2 \\ \delta_3 \end{bmatrix} = \mathbf{0}$$

$$\delta_1 = 1 \Rightarrow 0.4142k + 0.2929k\delta_2 = 0 \Rightarrow \delta_2 = -1.414$$

$$0.2929k\delta_2 + 0.4142k\delta_3 = 0 \Rightarrow \delta_3 = 1$$

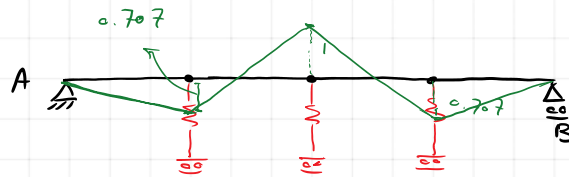
$$\delta = \begin{bmatrix} 1 \\ -1.414 \\ 1 \end{bmatrix} / 1.414$$

$$\rightarrow \delta = \begin{bmatrix} 0.707 \\ -1 \\ 0.707 \end{bmatrix}$$

$$P_{cr1} = 0.2929k$$

$$\delta = \begin{bmatrix} 0.707 \\ -1 \\ 0.707 \end{bmatrix}$$

Mode shape 1

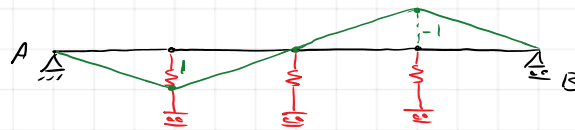


$$P_{cr2} = 0.5kL$$

$$A(k, 0.5k \cdot l, l) \rightarrow \begin{bmatrix} 0.0 & 0.5 \cdot k & 0 \\ 0.5 \cdot k & 0.0 & 0.5 \cdot k \\ 0 & 0.5 \cdot k & 0.0 \end{bmatrix}$$

$$\delta_1 = 1, 0.5k\delta_2 = 0 \Rightarrow \delta_2 = 0$$

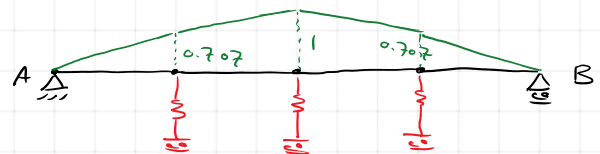
$$0.5k + 0.5k\delta_3 = 0 \Rightarrow \delta_3 = -1$$



$$P_{cr3} = 1.7071kL$$

$$\delta = \begin{bmatrix} 1 \\ 1.4142 \\ 1 \end{bmatrix} / 1.4142$$

$$\delta = \begin{bmatrix} 0.707 \\ 1 \\ 0.707 \end{bmatrix}$$



$$\begin{bmatrix} -2.4142 \cdot k & 1.7071 \cdot k & 0 \\ 1.7071 \cdot k & -2.4142 \cdot k & 1.7071 \cdot k \\ 0 & 1.7071 \cdot k & -2.4142 \cdot k \end{bmatrix} \delta = \mathbf{0}$$

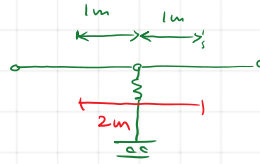
$$-2.4142k + 1.7071k\delta_2 = 0 \Rightarrow \delta_2 = 1.4142$$

$$1.7071k(1.4142) - 2.4142k\delta_3 = 0 \Rightarrow \delta_3 = 1$$

$$C_u = 10 \text{ kPa}$$

subgrade reaction coefficient: 100

$$\text{subgrade reaction: } k_s = A \cdot \frac{C_u}{l_{\text{eff}}}$$



$$k_{\text{spring}} = \underbrace{k_s}_{\frac{A \cdot C_u}{l_{\text{eff}}}} \cdot \underbrace{l_{\text{eff}}}_{2\text{m}} \cdot \text{Spacing}$$

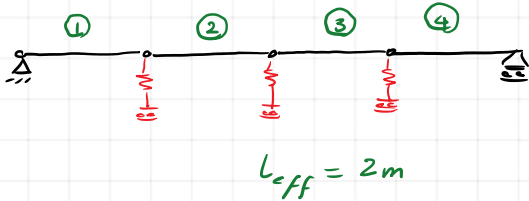
$$\Rightarrow k_{\text{spring}} = A \cdot C_u \cdot 2\text{m} = 200 C_u \text{ (m)}$$

$$C_u = 10 \text{ kPa} \rightarrow k_{\text{spring}} = 200 \times 10 \text{ kPa} = 2000 \frac{\text{kN}}{\text{m}}$$

$$P_{cr1} = 0.2929 KL = 0.2929 \times 2000 \frac{\text{kN}}{\text{m}} \times 2\text{m} = 1171.6 \text{ kN}$$

$$P_{cr2} = 2000 \text{ kN}$$

$$P_{cr3} = 6828 \text{ kN}$$



$$P_{cr} = \frac{\pi^2 EI}{l_{\text{eff}}^2} \quad E = 210 \text{ GPa}, I = 1.402 \times 10^6 \text{ mm}^4$$

$$l_{\text{eff}} = 2\text{m}$$

$$P_{cr} = \frac{\pi^2 \times 210 \text{ GPa} \times 1.402 \times 10^6 \text{ mm}^4}{(2\text{m})^2} = 726 \text{ kN}$$

First four modes will be 726 kN 1171.6, 2000, 6828

$$\Rightarrow P_{cr} = 8104 \text{ kN} \quad 1171.6, 2000, 6828, 8104$$

$$\Rightarrow P_{cr} = 3050 \text{ kN}$$

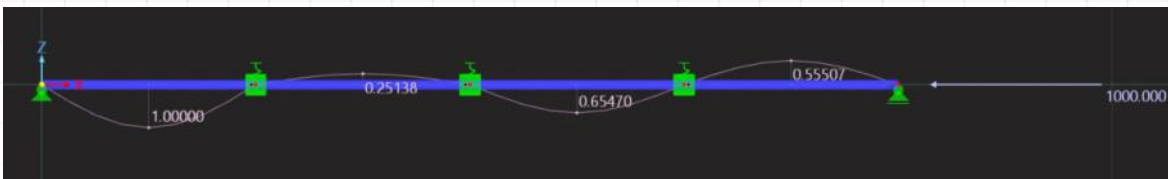
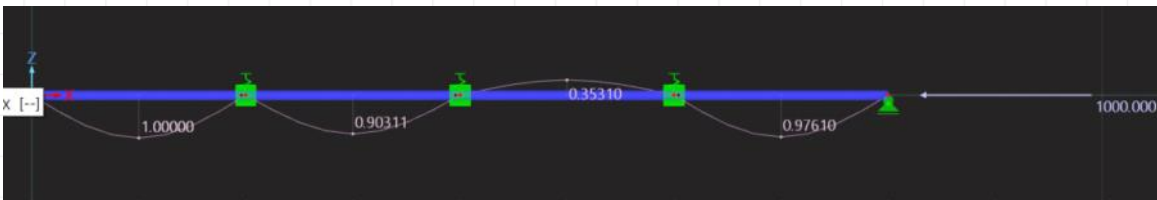
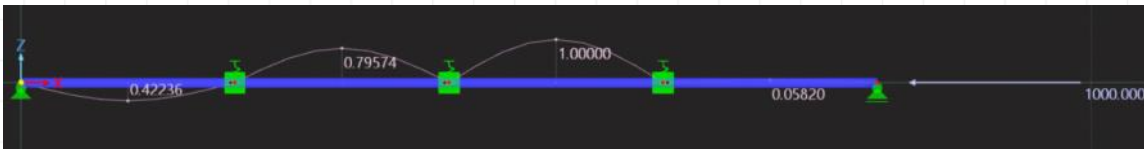
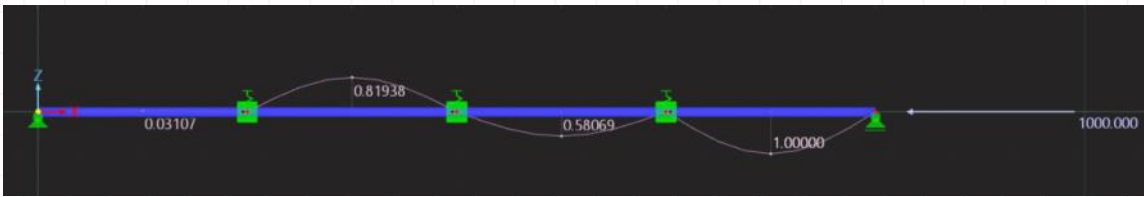
$$1171.6, 2000, 3050, 3050, 3050, 3050, 6828$$

Profile	Drawing	Profile dimensions		Area properties				Second moment of area I [ $\times 10^6 \text{ mm}^4$ ]
		Ext. Diameter D [mm]	Wall thickness t [mm]	Weight m [kg/m]	External perimeter P [m]	Area A [ $\text{mm}^2$ ]	Shear area A <sub>v</sub> [ $\text{mm}^2$ ]	
CHS 88.9 / 6.3	dx1	88.9	6.3	12.8	0.279	1635	1041	1.402

Profile	Drawing	Profile dimensions		Area properties				Second moment of area I [ $\times 10^6 \text{ mm}^4$ ]
		Ext. Diameter D [mm]	Wall thickness t [mm]	Weight m [kg/m]	External perimeter P [m]	Area A [ $\text{mm}^2$ ]	Shear area A <sub>v</sub> [ $\text{mm}^2$ ]	
CHS 168.3 / 10	dx1	168.3	10.0	39.0	0.529	4973	3166	15.64

Profile	Drawing	Profile dimensions		Area properties				Second moment of area I [ $\times 10^6 \text{ mm}^4$ ]
		Ext. Diameter D [mm]	Wall thickness t [mm]	Weight m [kg/m]	External perimeter P [m]	Area A [ $\text{mm}^2$ ]	Shear area A <sub>v</sub> [ $\text{mm}^2$ ]	
CHS 139.7 / 6.3	dx1	139.7	6.3	20.7	0.439	2640	1681	5.886

The first four modes:



Mode No.	Critical Load Factor f [-]
1	0.718
2	0.718
3	0.718
4	0.718

$$P_{cr} = 0.718 \times P_{\text{applied}} = 718 \text{ kN}$$



1.172
2.000

$$P_{cr} = 1.172 \times P_{\text{applied}} = 1172 \text{ kN}$$



$$P_{cr} = 2 \times P_{\text{applied}} = 2000 \text{ kN}$$



$$P_{cr} = 6.828 \times P_{\text{applied}} = 6828 \text{ kN}$$