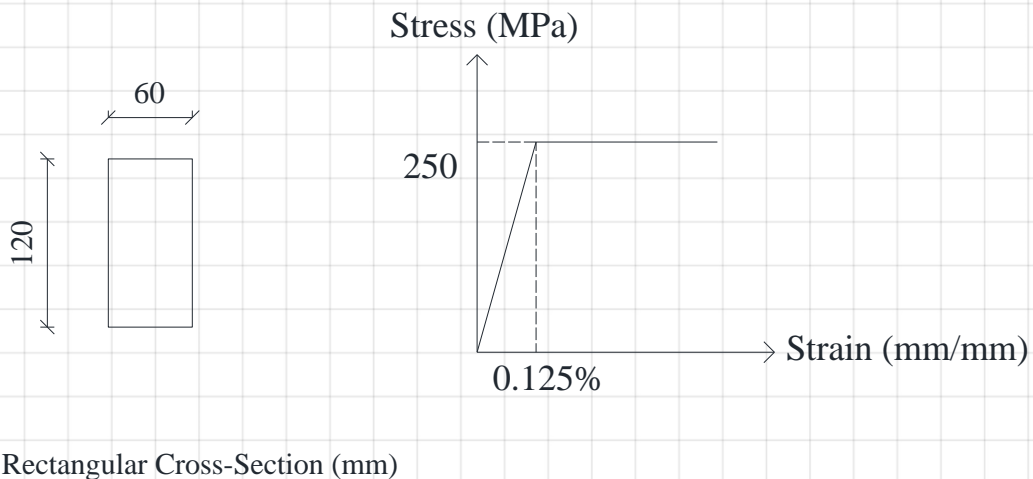
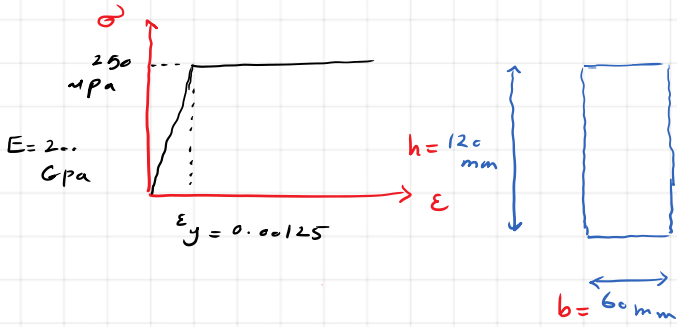


The shown rectangular cross-section is made of an elastic, perfectly plastic material.

- Determine the elastic and plastic bending moment.
- If the cross-section is under a bending moment of 50kN.m, what is the depth of the elastic section?
- If the applied bending moment is removed from the cross-section, determine the residual stress on the cross-section.
- Derive the relation between the bending moment and the maximum strain of the cross-section on its edge.
- Sketch the function of bending moment with the dependent variable of the furthest edge strain. Clearly show the linear and non-linear behavior of the cross-section.
- Determine the permanent strain of the furthest cross-section edge considering two methods. First, with the derived graph from part e. Second, with the stress-strain diagram.
- Cross-check your calculation with the permanent strain.



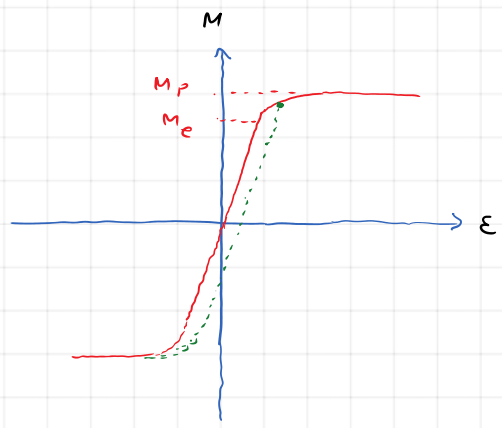


$$W_{el} = \frac{bh^2}{6} = 144000 \text{ mm}^3$$

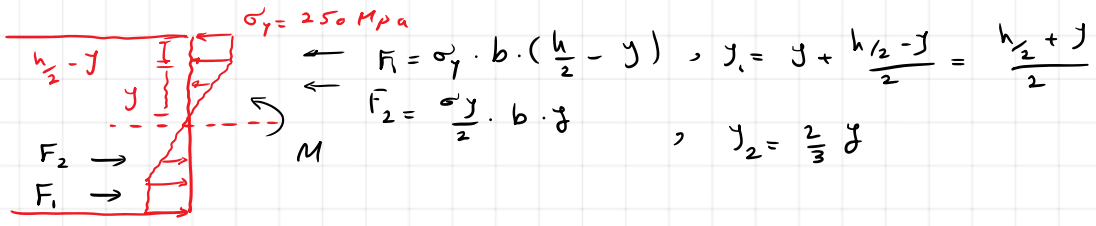
$$W_{pl} = \frac{bh^2}{4} = 216000 \text{ mm}^3$$

$$M_{el} = \sigma_y \cdot W_{el} = 36 \text{ kN}\cdot\text{m}$$

$$M_{pl} = \sigma_y \cdot W_{pl} = 54 \text{ kN}\cdot\text{m}$$



$M_{el} < M = 50 \text{ kN}\cdot\text{m} < M_{pl} \rightarrow$  partially plastic



$$M = \sum F_i \cdot y_i = 2 \left[ F_1 \cdot y_1 + F_2 \cdot y_2 \right] \Rightarrow$$

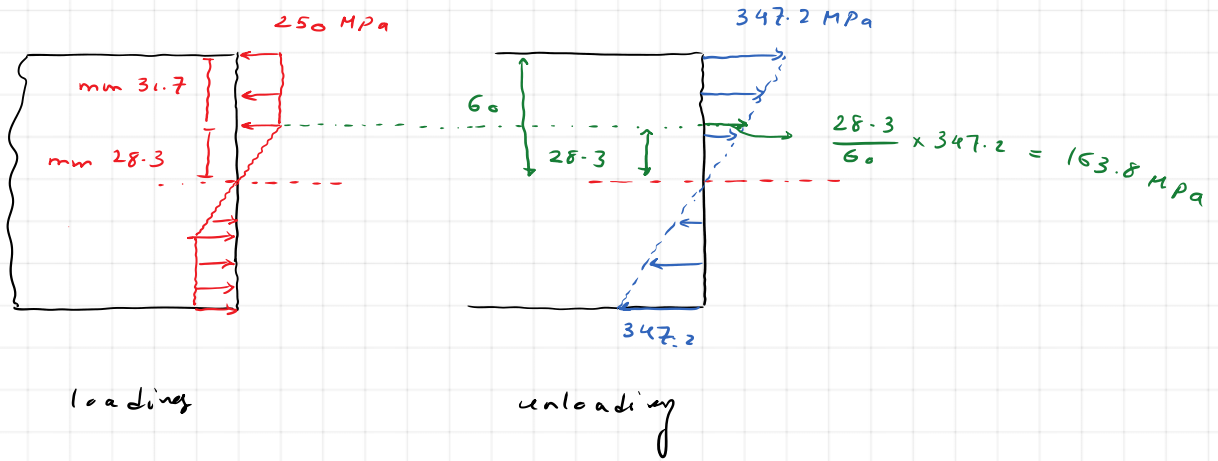
$$50 \text{ kN}\cdot\text{m} = 2 \left[ \sigma_y \cdot b \cdot (h/2 - y) \cdot \left( \frac{h/2 + y}{2} \right) + \frac{\sigma_y}{2} \cdot b \cdot y \cdot \frac{2}{3} y \right]$$

$\sigma_y = 250 \text{ MPa}$ ,  $b = 60 \text{ mm}$ ,  $h = 120 \text{ mm}$

$$\rightarrow y = 28.3 \text{ mm}$$

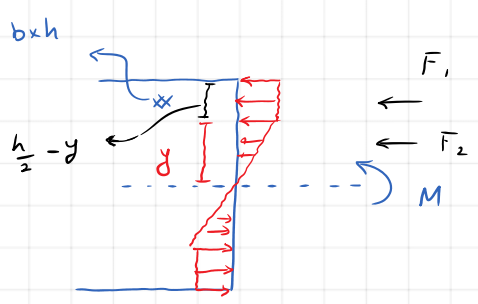
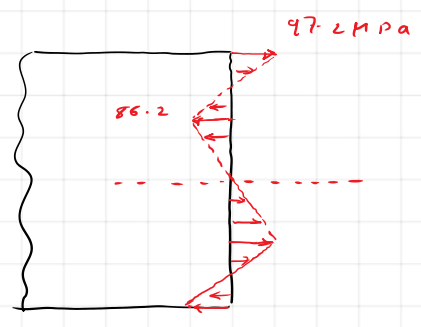
unloading:

$$\sigma = \frac{M \cdot y}{I} = \frac{M}{W_{el}} \rightarrow \sigma = \frac{50 \times 10^6 \text{ N}\cdot\text{mm}}{144000 \text{ mm}^3} = 347.2 \text{ MPa} < \underbrace{\sigma_{y(+)} + \sigma_{y(-)}}_{500 \text{ MPa}}$$



$$-250 \text{ MPa} + 347.2 \text{ MPa} = 97.2 \text{ MPa}$$

$$-250 \text{ MPa} + 163.8 \text{ MPa} = -86.2 \text{ MPa}$$

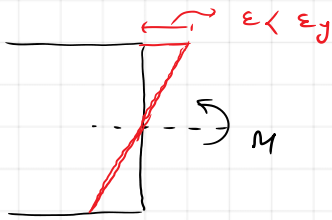


$$M_{el} = \sigma_y \cdot \frac{bh^2}{6} \quad \rightarrow \quad M_{pl} = \sigma_y \cdot \frac{bh^2}{4}$$

$$F_1 = b \cdot \left(\frac{h}{2} - y\right) \cdot \sigma_y \quad y_1 = y + \left(\frac{h}{2} - y\right) \div 2 = y + \frac{h/2 - y}{2} = \frac{h/2 + y}{2}$$

$$F_2 = b \cdot y \cdot \frac{\sigma_y}{2} \quad y_2 = \frac{2}{3} \cdot y$$

$$M = \sum F_i \cdot y_i = 2 \left[ F_1 \cdot y_1 + F_2 \cdot y_2 \right] = 2 \left[ \sigma_y \cdot b \left(\frac{h}{2} - y\right) \left(\frac{h/2 + y}{2}\right) + \sigma_y \cdot \frac{1}{2} \cdot b \cdot y \cdot \frac{2}{3} y \right]$$



(Strain)

$$M < M_{el}$$

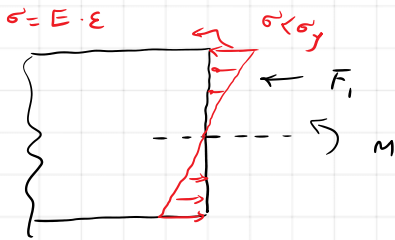
$$\sigma = E \cdot \varepsilon$$



(Stress)

$$M_{el} < M < M_{pl}$$

$$\frac{\varepsilon}{h/2} = \frac{\varepsilon_y}{j} \Rightarrow \boxed{j = \frac{h}{2} \cdot \frac{\varepsilon_y}{\varepsilon}} \quad \text{I}$$



(Stress)

$$(M < M_{el})$$

$$M = F_i \cdot j = 2 \cdot \frac{1}{2} \sigma \cdot \frac{h}{2} \cdot b \cdot \frac{h}{3} \cdot \frac{h}{2} \cdot 2 = \sigma \cdot \frac{bh^2}{6}$$

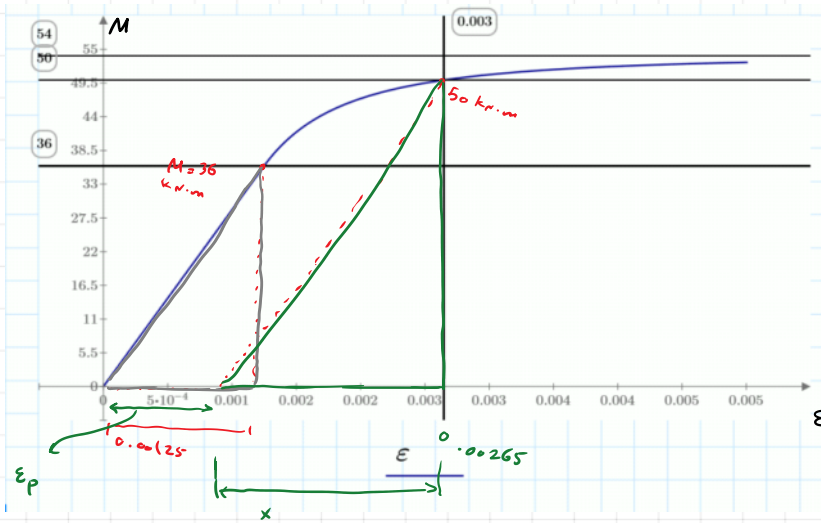
$$M = E \cdot \varepsilon \cdot \frac{bh^2}{6}$$

$$M < M_{el}$$

$$M_{el} < M < M_{pl}$$

$$M(b, h, \sigma_y, y) := 2 \cdot \sigma_y \cdot b \cdot \left( \frac{1}{2} \cdot \left( \frac{h}{2} - y \right) \cdot \left( \frac{h}{2} + y \right) + \frac{1}{3} \cdot y^2 \right)$$

$$\text{I} \quad j = \frac{h}{2} \cdot \frac{\varepsilon_y}{\varepsilon}$$



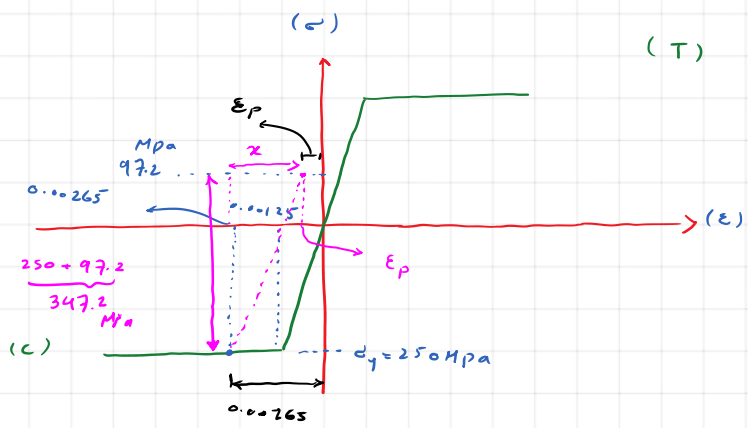
$M(\epsilon) \text{ (kN}\cdot\text{m)}$

$$\frac{50}{x} = \frac{36}{0.00125}$$

$$x = 0.00174$$

$$\epsilon_p = 0.00265 - 0.00174$$

$$\epsilon_p = 0.00091$$



$$\frac{347.2}{x} = \frac{250}{0.00125}$$

$$x = 0.00174$$

$$\epsilon_p = 0.00265 - x = 0.00091$$